



ICOMOS New Zealand and
Historic Places Aotearoa Conference
12 November 2022

SEISMIC RESILIENCE – CHALLENGES FOR BUILT HERITAGE
CASE STUDIES

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SEISMIC RESILIENCE – CHALLENGES FOR BUILT HERITAGE

5 CASE STUDIES:

1. **AGGS:** Implications of previous seismic intervention for **maintenance**
2. **UoO CAPS Building:** **Redevelopment** triggering seismic intervention
3. **Gisborne Cenotaph:** Major repairs and upgrading after 2007 **seismic event**
4. **Seddon Memorial:** **Conservation work** triggering seismic upgrading
5. **Domain Wintergardens:** Seismic upgrading (removal from EPB Register) **triggering major replacement of fabric**

Case Study 1: Auckland Girls Grammar School

Implications of seismic intervention for maintenance

2006 – asked to assess extensive deterioration of brickwork and defects internally

- Category 1 Heritage Building
- built of traditional cavity brick construction
- parapet walls / pitched slate roofs / cast iron downpipes





- extensive efflorescence (salts) externally
- water penetration – damp plaster internally
- failure of plaster internally (salt contaminated)
- blocked / broken downpipes - leaking

1990s seismic retrofit



Steel restraints to gable walls



Structural steel fitted at flashing /upstands level



52 penetrations to slate roof reliant on sealant

Cavities of brickwork had been filled with high strength concrete

Shotcrete (sprayed concrete on a galvanised steel mesh) applied to interior walls

Floor diaphragms installed

DEFECTS CAUSED BY:

1. Water penetration:
 - Blocked, broken rainwater pipes /Blocked drains;
 - Leaking parapet gutter linings;
 - Structural steelwork pierced flashings and upstands;Problem - No longer a drainable cavity
2. Soluble salts migration from cementitious grout filled cavity – causing failure of brickwork and the plaster

SOLUTION: Not maintenance....

but a major project and then continue to monitor.

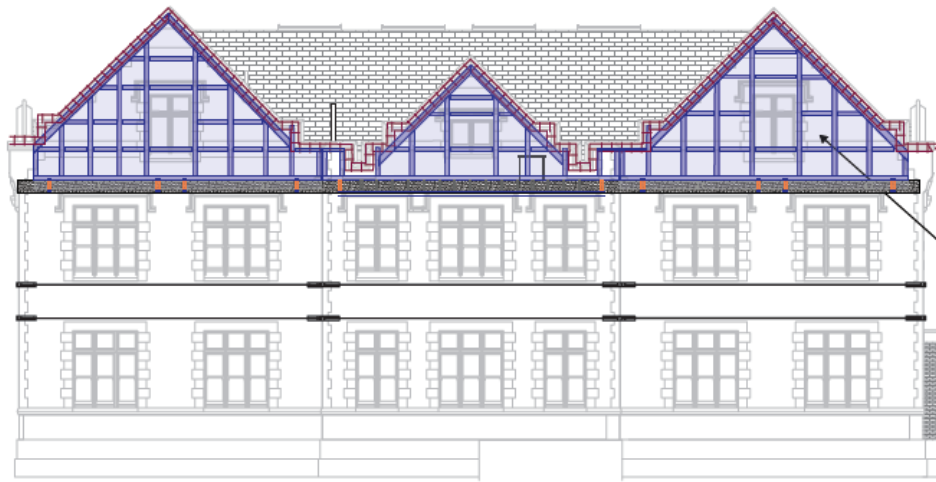
Seismic upgrade 15years old – caused major implications for health and maintenance of this building



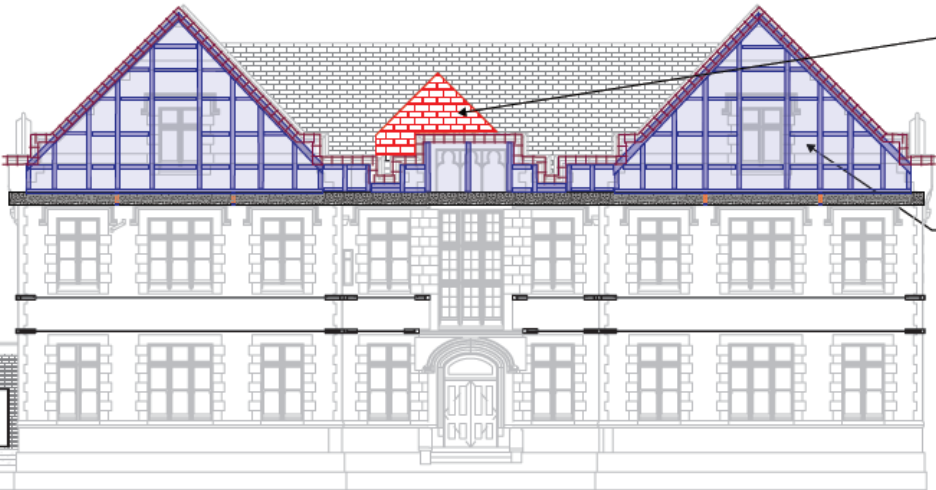
Case Study 2: University of Otago Consumer & Applied Sciences Building: **Redevelopment** triggering seismic intervention



- 1918, solid mass masonry - URM;
- Category I – 1 of 6 significant heritage precinct buildings - rare examples of 19th /early 20th century gothic revival university architecture;
- Campus redevelopment – extend the CAPS by adding a new building with a link building;
- New building – to be designed to 100%NBS;
- CAPS seismically assessed as 10-15%NBS for IL2;
- University proceeded to developed design stage for both the new building and the CAPS – brief to the engineer was to achieve 100%NBS at IL2;



West Elevation



East Elevation

Resource Consent Stage:

Salmond Reed Architects asked to review the proposal and the impact on the heritage building just prior to submitting application.

The Proposal

- Dismantling all stone gable walls including windows
- New concrete ring beam
- Grouting of soft fill to the solid masonry walls (stone /brick)
- Rebuilding the gable walls - lightweight construction
- Removal of internal of timber walls replace with concrete
- External post tensioning
- Partial underpinning



Case Study 2

Rebuilding the gable walls - lightweight construction??

Otago University's Seismic Strengthening Policy:

- “ that all existing buildings, which are defined as Earthquake Prone, should be brought to at least **34%NBS**”.
- “Heritage Buildings will be strengthened in accordance with the targets above, and with due respect to their heritage fabric and character”.
- “All buildings, that are Earthquake Prone will have priority for strengthening to at least **34%NBS** in accordance with Building Act timeframes”.

Otago University's Campus Master Plan:

Chapter 6: Design guidelines - Historic core

- “The restoration of this fine group of neo-gothic buildings should focus on stripping away later accretions to the interiors, to reveal the original spaces, architecture and ornament”.

Dunedin CC District Plan

“prioritise protection of **heritage values over compliance** with other performance standards where there is a **conflict**”

A2.1.1.3 Principal threats to values:

- d. The removal of **original materials and features** from heritage buildings

Final Assessment:

The proposal, as currently presented will:

- cause significant loss of valuable heritage fabric;
- **be in contravention of policy statement for achieving of min 34%NBS;**
- adversely affect the integrity and authenticity;
- not satisfactorily balance engineering performance with heritage preservation and sustainability;
- result in the upper third of the building, with its most significant architectural features ending up as **skin deep replicas**, that have a shorter renewal period, and so will increase life-cycle costs of the building;
- ultimately devalue the heritage asset

Recommendation

University review alternative structural options between a target of min 34%NBS and 67%NBS seismic resilience, in order to compare the adverse effects on the heritage fabric versus the overall benefits.

Final Outcome:

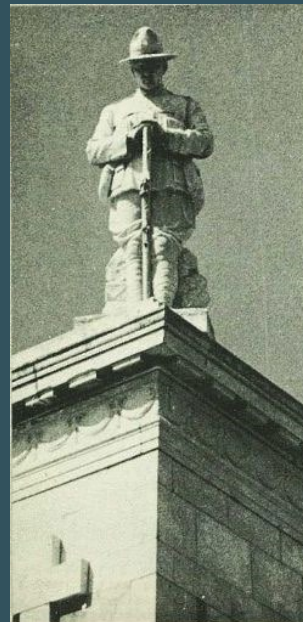
Revised substantially – reduced scheme to achieve 67%NBS at IL2

Case Study 3: Gisborne Cenotaph

Repairs and upgrading following a **seismic event**



In 2007, an earthquake struck the Gisborne region, causing significant damage to a number of buildings in the central city. The Cenotaph itself was also damaged, with the upper shaft bearing the figure of a soldier dislocated from the base and moved about 65mm to the north-west. This is not the first instance of earthquake effects on the structure – an earlier event resulted in the figure of the soldier being rotated about 45 degrees.



Initial repair proposal being advocated:

Demolish and rebuild – around a new concrete core

Led to a long process of deliberations about the impact and costs



Case Study 3

Investigations – Multi-disciplinary Team



Case Study 3

Investigations – finding the hollow core



Figure 1: Photomicrograph, typical field of view of concrete from interior of Gisborne Cenotaph, showing fine aggregate particles of calcareous and other mudstone set in limey paste (higher relief, pale grey). Note oxidised rim to clast at top left. Colourless areas have lost friable paste during specimen preparation. Transmitted light. Scale: width of view is 6 mm.

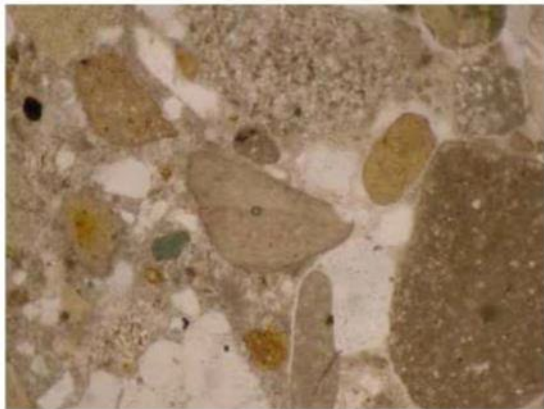
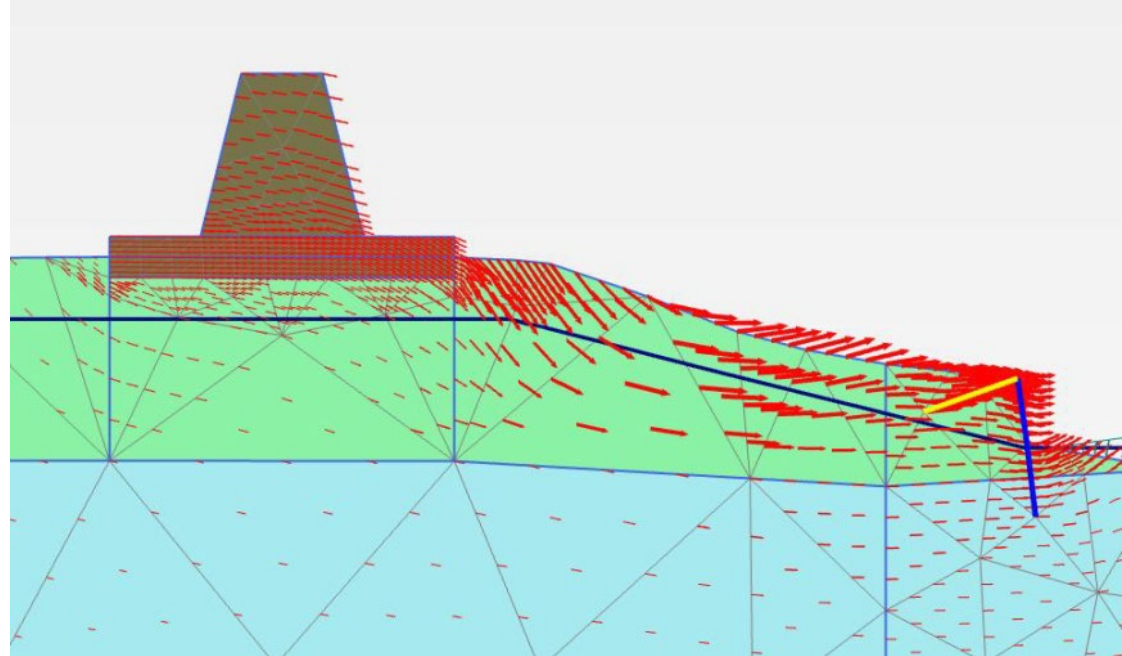
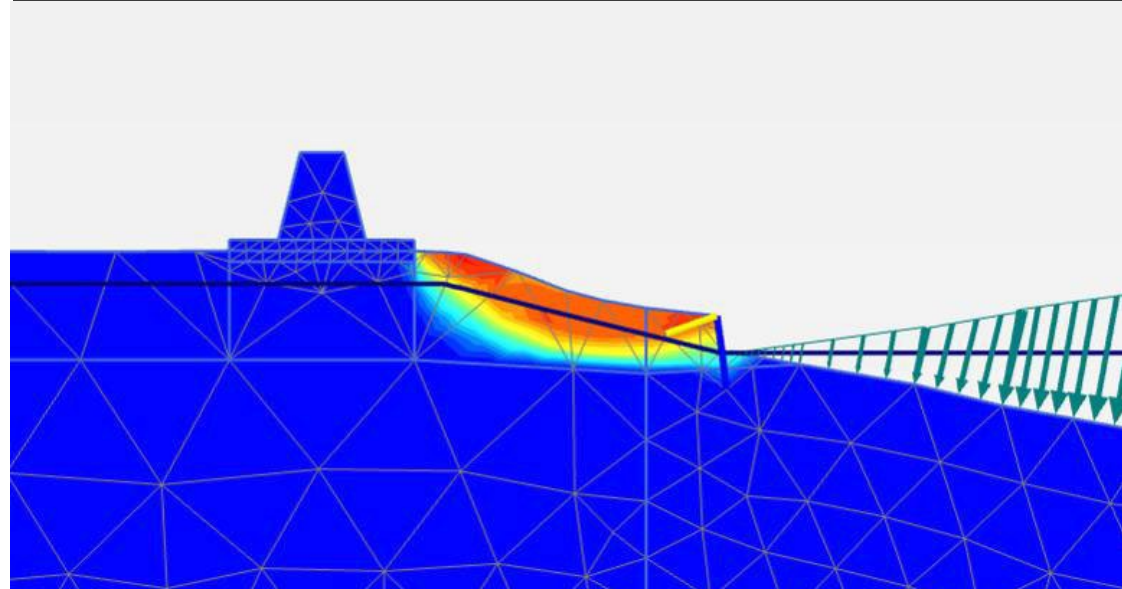


Figure 1: Photomicrograph, typical field of view of concrete from interior of Gisborne Cenotaph, showing fine aggregate particles of calcareous and other mudstone set in limey paste (higher relief, pale grey). Colourless particles are volcanic quartz and molluscan shelly material. Transmitted light. Scale: width of view is 6 mm.

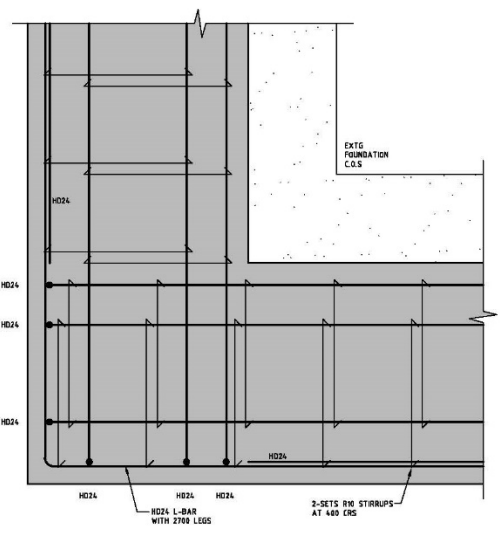


- Removal and conservation repair of marble steps;
- Significant underpinning and connect to the existing piles;
- Repairs and repositioning of the shaft;
- Use of the void to provide seismic strength – meccano style stainless steel frame;
- Improve connection of the statue to the shaft

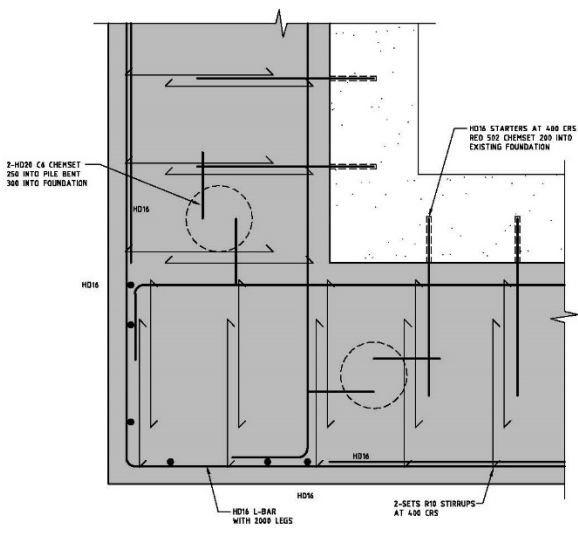
NO	REVISION	BY	DATE
A	FOR CONSTRUCTION	TOP	01/11/13

Copyright of this drawing is vested in Spencer Holmes Ltd.
The Contractor shall verify all dimensions on site.

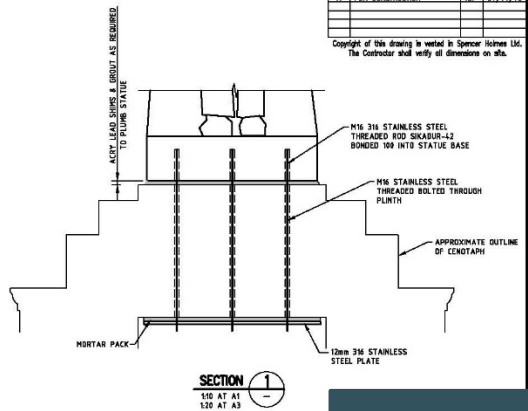
STRUCTURAL - CIVIL - FIRE



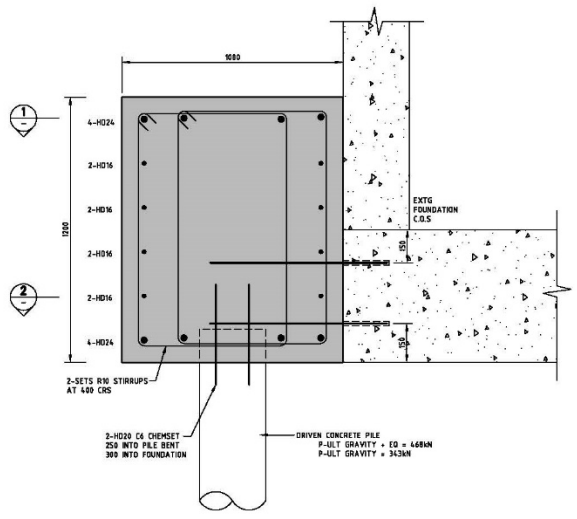
SECTION 1
120 AT A1
140 AT A3



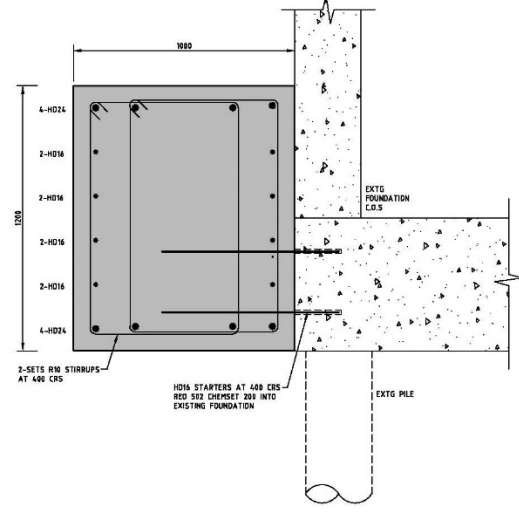
SECTION 2
120 AT A1
140 AT A3



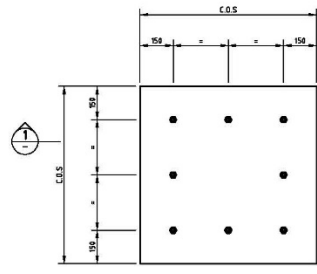
SECTION 1
110 AT A1
110 AT A3



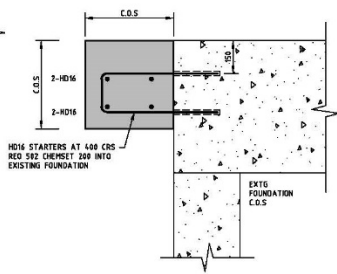
DETAIL A
120 AT A1
140 AT A3



DETAIL B
120 AT A1
140 AT A3



STATUE BASE FIXING PLAN
SCALE 1:10 AT A1, 1:20 AT A3



TYPICAL UPPER FOUNDATION SECTION
SCALE 1:10 AT A1, 1:20 AT A3

Case Study 3

Strengthening: Underpinning and Statue Fixing



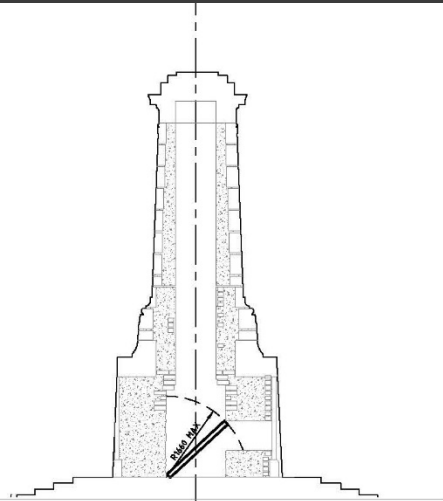
Case Study 3

Underpinning – Non ferrous reinforcing bars



Case Study 3

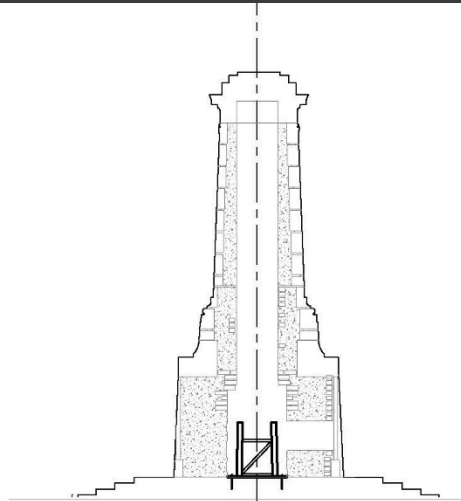
Using the Void for Seismic Resilience



**CENOTAPH SECTION SHOWING
APPROX CLEARANCE FOR
INSTALLATION OF MEMBERS**

1
51

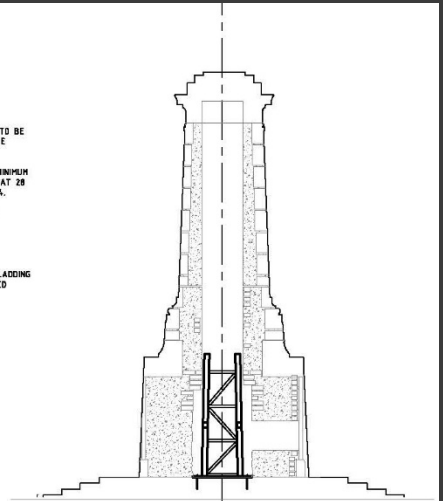
SCALE 1:50 AT A1 (1:100 AT A3)



**CENOTAPH SECTION SHOWING
STEEL WORK SECTION ONE**

1
51

SCALE 1:50 AT A1 (1:100 AT A3)



**CENOTAPH SECTION SHOWING
STEEL WORK SECTION TWO**

1
51

SCALE APPROX 1:50 AT A3

SPECIFICATION

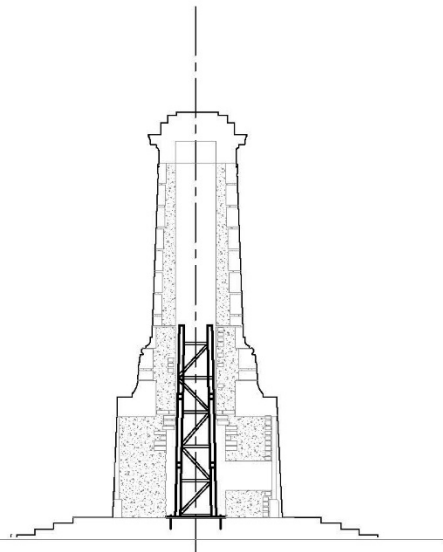
MATERIALS AND WORKMANSHIP ARE TO BE OF THE STANDARDS LAID DOWN IN THE RELEVANT NZS.

FOUNDATION CONCRETE TO HAVE A MINIMUM COMPRESSIVE STRENGTH OF 30 MPa AT 28 DAYS IN ACCORDANCE WITH NZS 3104.

ALL STEEL WORK TO BE GRADE 316 STAINLESS STEEL.

ALL BOLTS TO BE GRADE A4-316 STAINLESS STEEL.

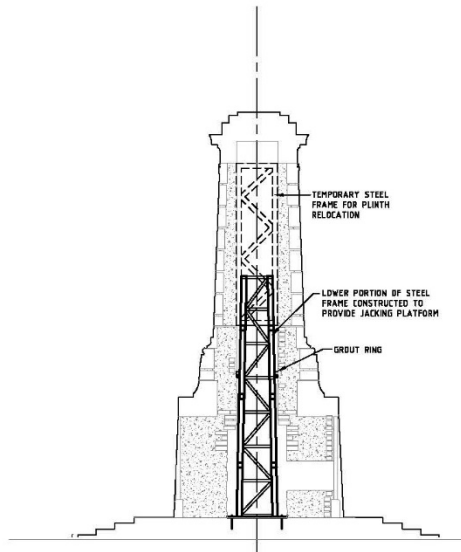
ALL WORK IN REGARD TO MARBLE CLADDING TO BE SUPERVISED BY SALMOND REED ARCHITECTS



**CENOTAPH SECTION SHOWING
STEEL WORK SECTION THREE**

1
51

SCALE 1:50 AT A1 (1:100 AT A3)



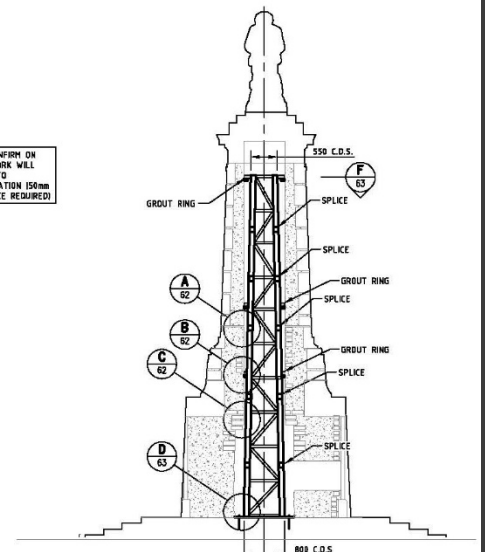
**CENOTAPH SECTION SHOWING
STEEL WORK SECTION FOUR**

1
51

SCALE 1:50 AT A1 (1:100 AT A3)

NOTES:

CONTRACTOR TO CONFIRM ON SITE THAT STEELWORK WILL FIT IN VOID PRIOR TO COMMENCING FABRICATION (50mm MIN. WALL CLEARANCE REQUIRED)



**CENOTAPH SECTION SHOWING
STEEL WORK INPLACE**

1
51

SCALE 1:50 AT A1 (1:100 AT A3)



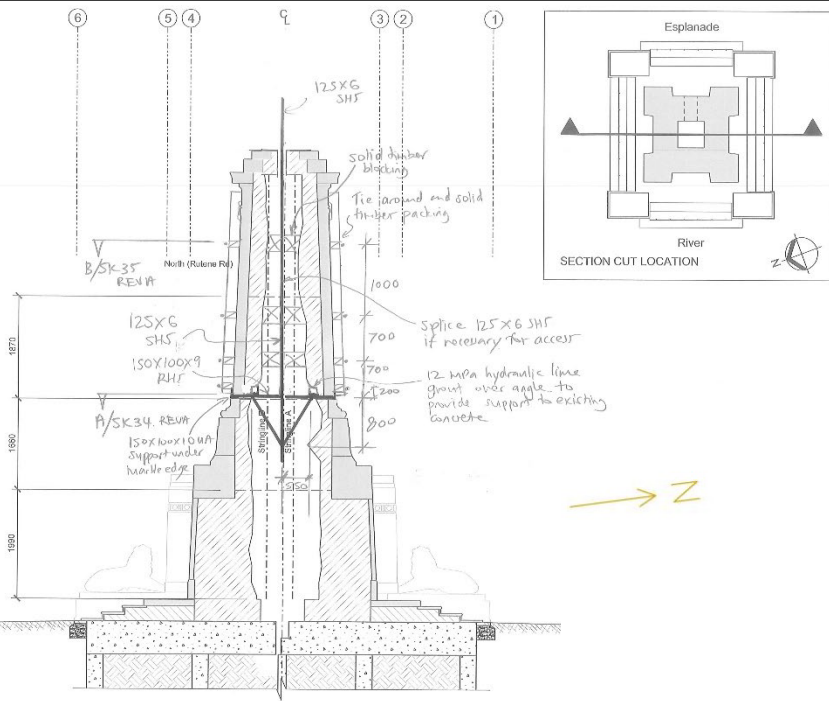
Case Study 3

Void Structural Frame



Case Study 3

Repositioning the shaft



EXISTING SECTION A-A
SCALE 1:50

LIFTING LUG TO ENGINEERS SPEC & DISTANCE FROM TOP OF MONUMENT

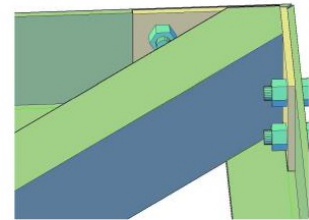
MAST SUPPORT EA 60x6 SIT ON TOP OF MONUMENT WHILE WORKER INSTALLS SOLID TIMBER BLOCKING

M16 BNV

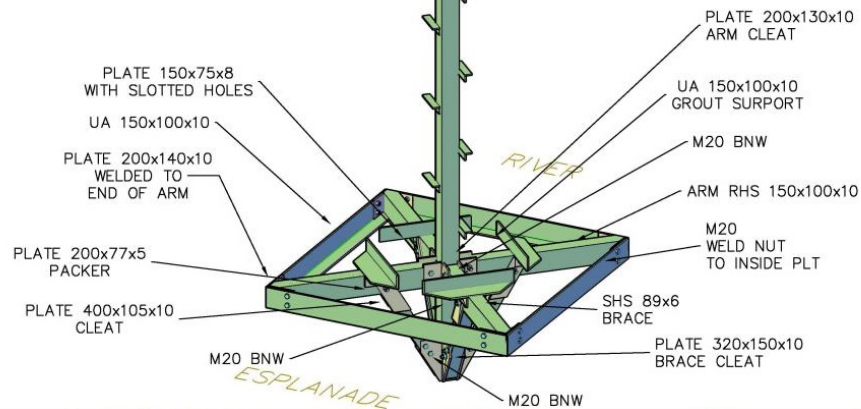
PLATE 130x80x10 SLOTTED HOLES 60x18

MAST SHS 125x6

STEP EA 50x6 @ 300 APART FOR WORKER TO INSTALL SOLID TIMBER BLOCKING



TYPICAL CORNER DETAIL



APPROVA

Case Study 3

Lifting and Repositioning the Shaft



Case Study 3

Completion for Anzac Celebrations 2015

Case Study 4: Seddon Memorial, Wellington

Repairs and maintenance **triggering upgrading**





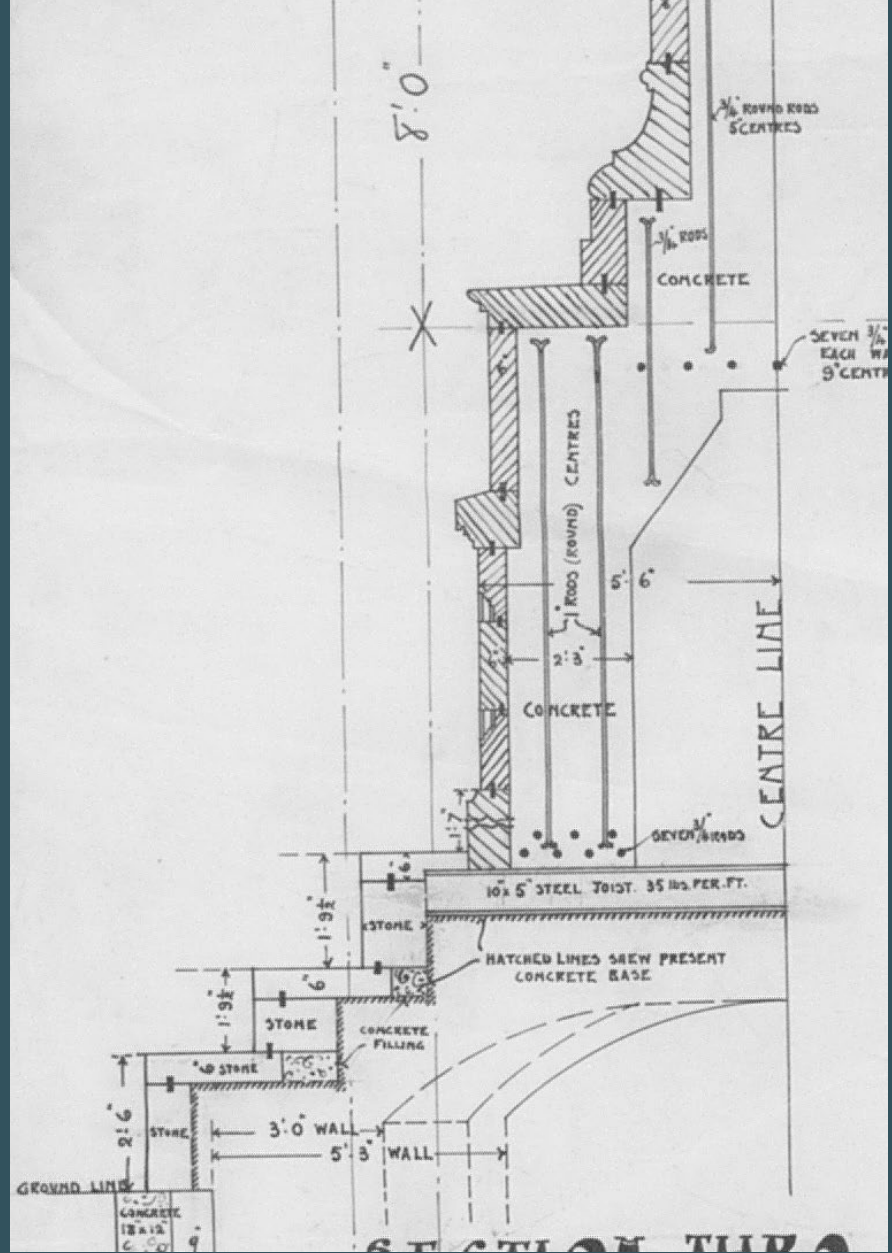
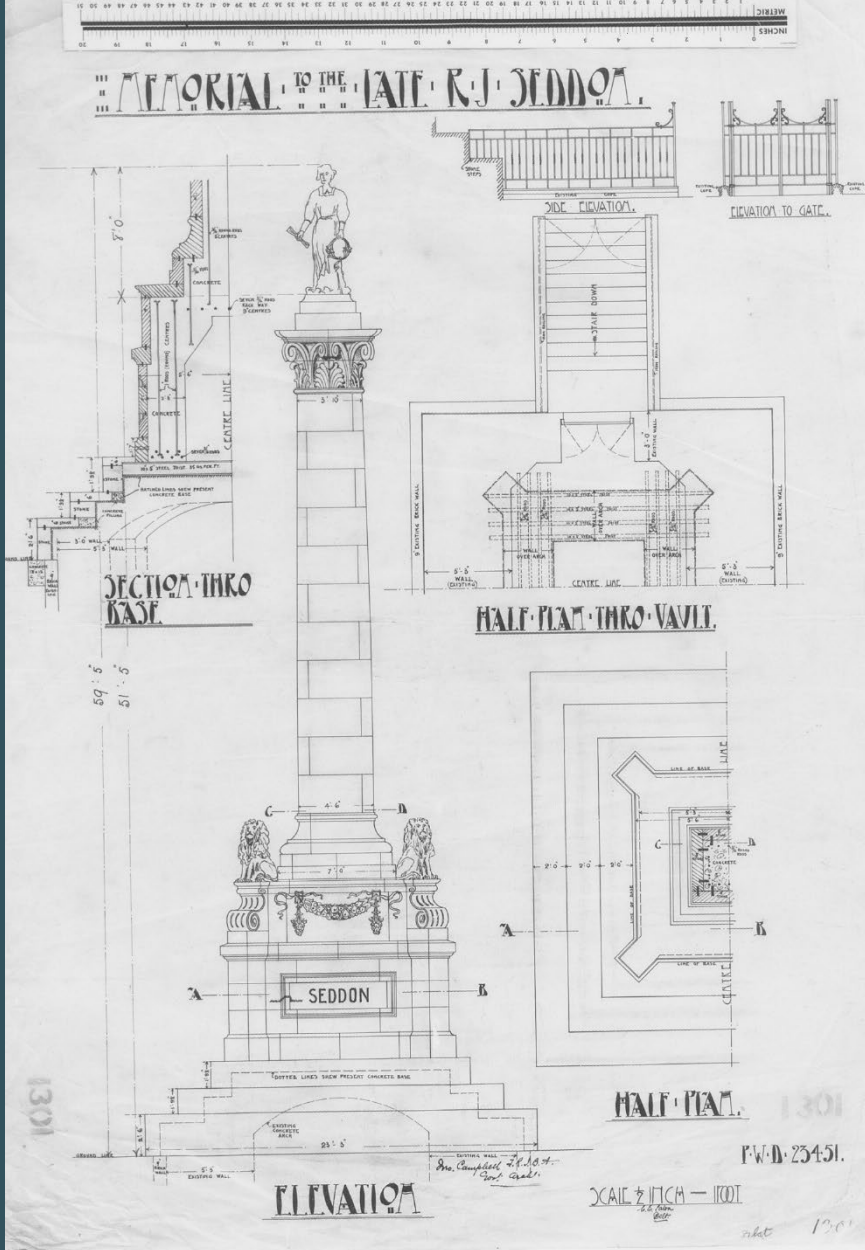
Case Study 4

Concerns about condition and water penetration



Case Study 4

Corrosion staining/water leaching – first noted 1930s



Case Study 4

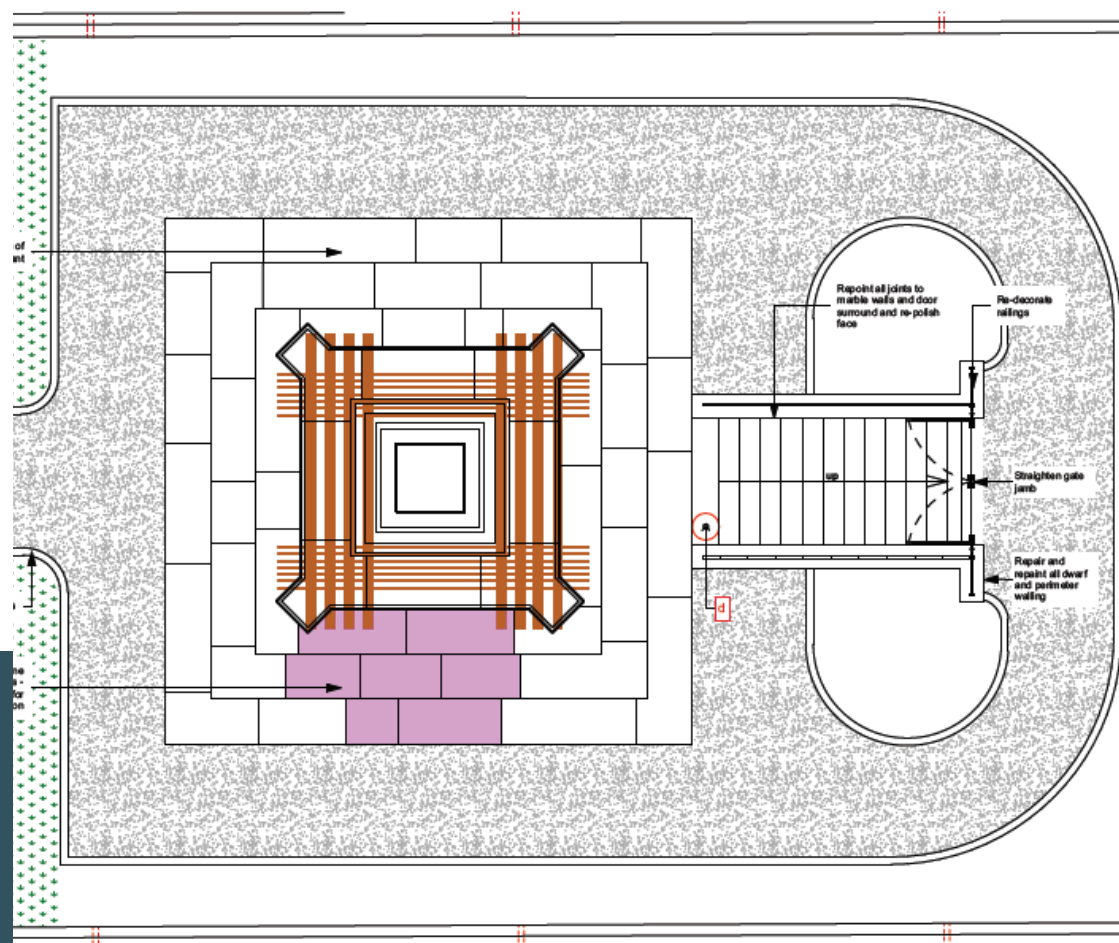
Research – original drawing / history of condition



Seddon Memorial Wellington

FINAL ISSUE

Conservation Statement
and Recommended
Scope of Repairs



Key Recommendations:

1. DSA be undertaken;
2. Physical investigations - reason for corrosion and establish condition and size of the void

Assessed - designation of IL2

Structural weaknesses:

- Statue potential for toppling;
- Column anchorage to plinth 15% (Critical);
- Column bending capacity 25%;
- Column pedestal overturning resistance 30%;
- Crypt overturning resistance 100%;

Classified - very high risk

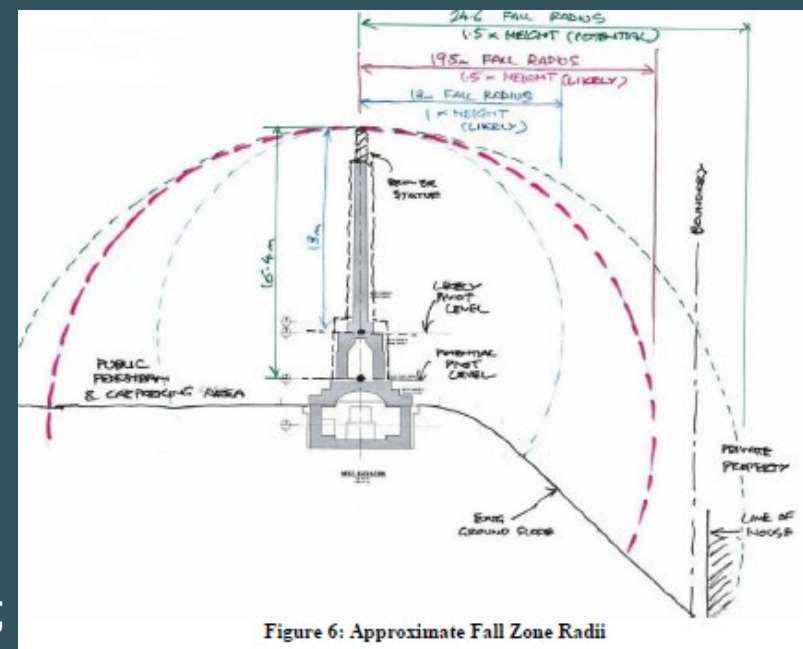


Figure 6: Approximate Fall Zone Radii

Option 1: Height Reduction of column

Reduction in height to 12.5m - 38%NBS

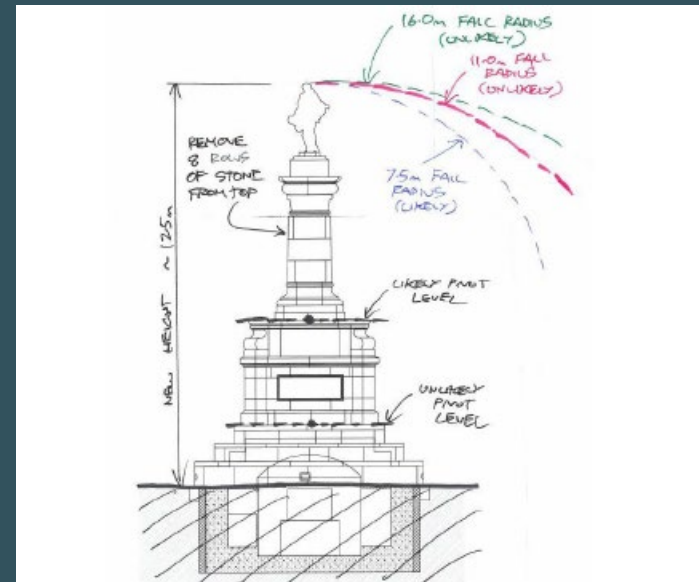


Figure 8 (Left): Reduced likely fall radius range to 7.5-11.0m, seismic rating 38%NBS(IL2)

Reduction in height to 10.5m - 100%NBS

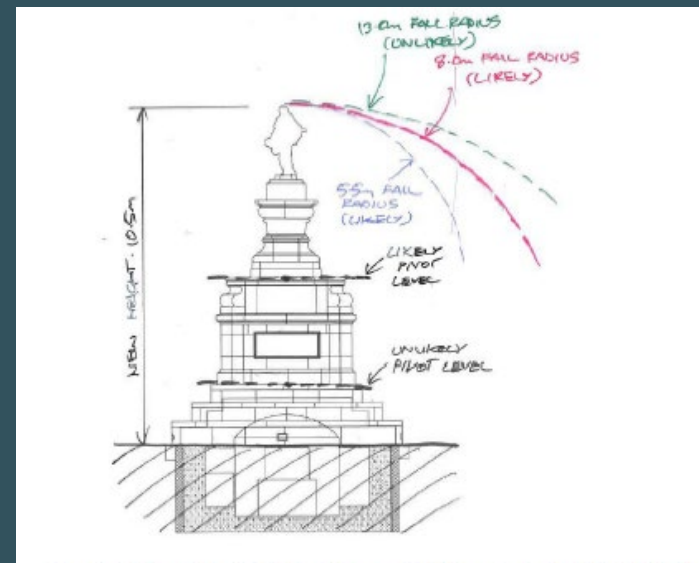
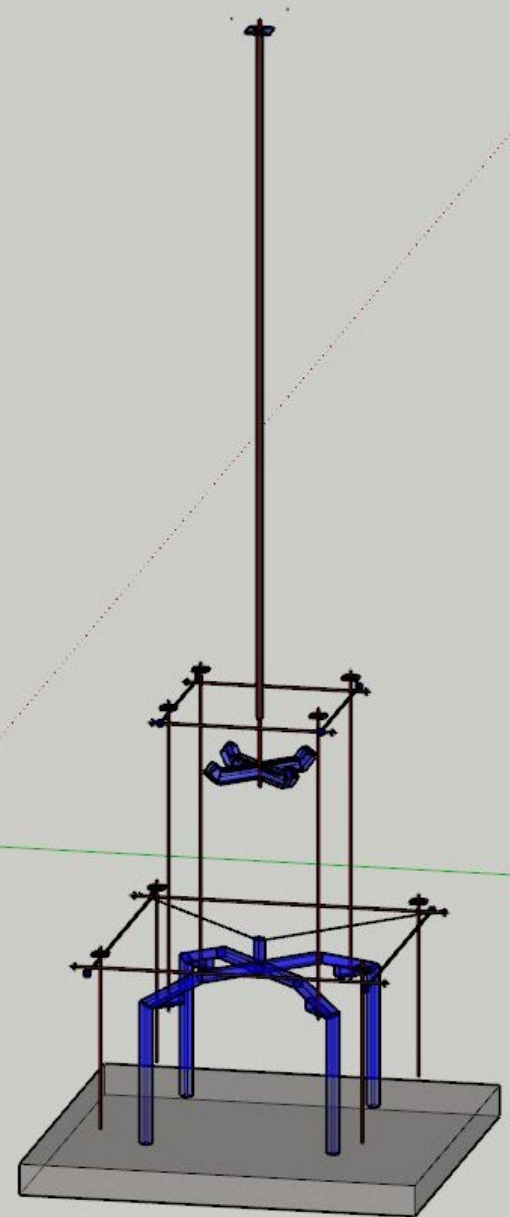


Figure 9 (Right): Reduced likely fall radius range to 5.5-8.0m, seismic rating >100%NBS(IL2)



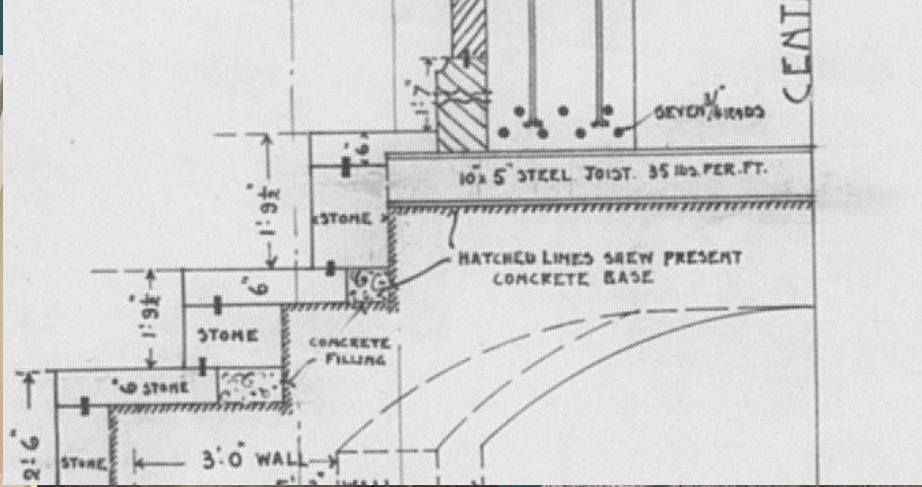
Case Study 4

Option 2: Strengthening to min 34%NBS



Case Study 4

Stage 1: Remove highest risk – the statue & investigate fixings



Case Study 4

Investigating the corrosion



Case Study 4

Breaking through to the void



Case Study 4

Saturated Void



Case Study 4

Corrosion ? Never assume anything!



Case Study 4

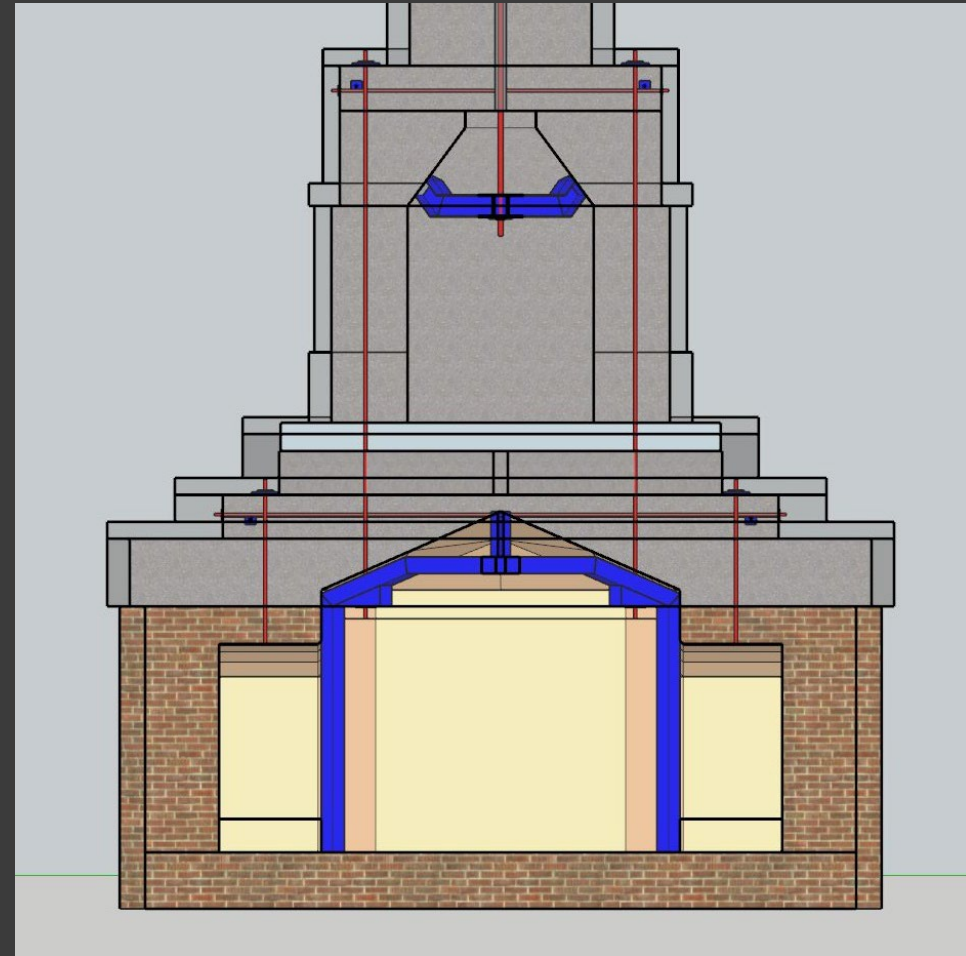
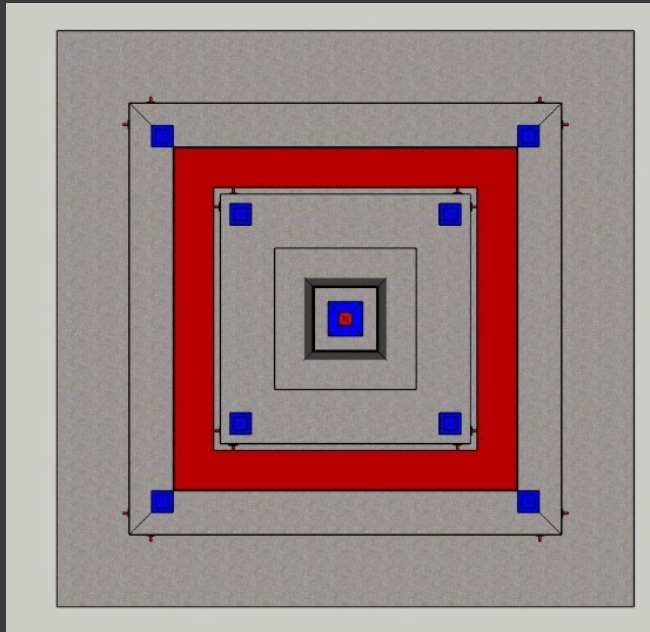
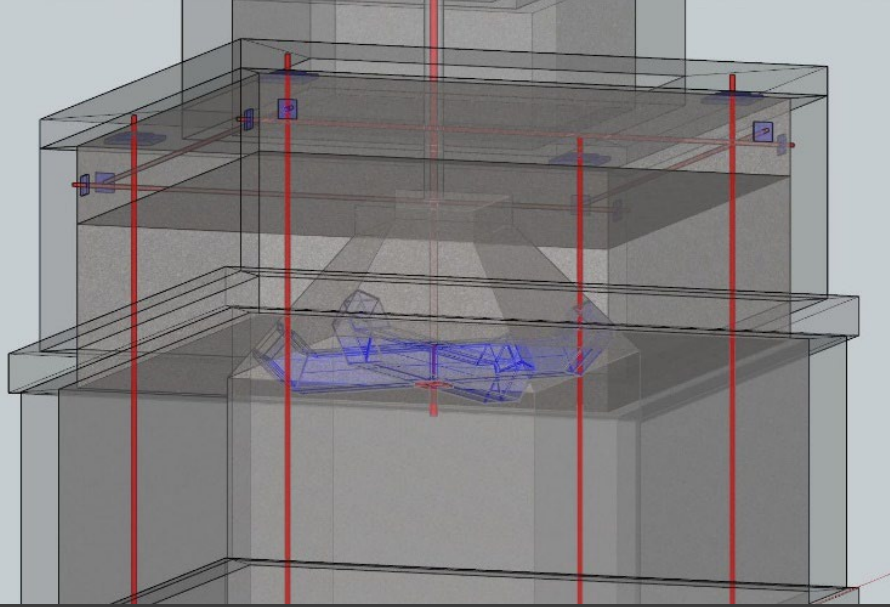
Rotted timber – Tannins causing staining



Figure 1: Core sample 4-21/563A before and after testing.

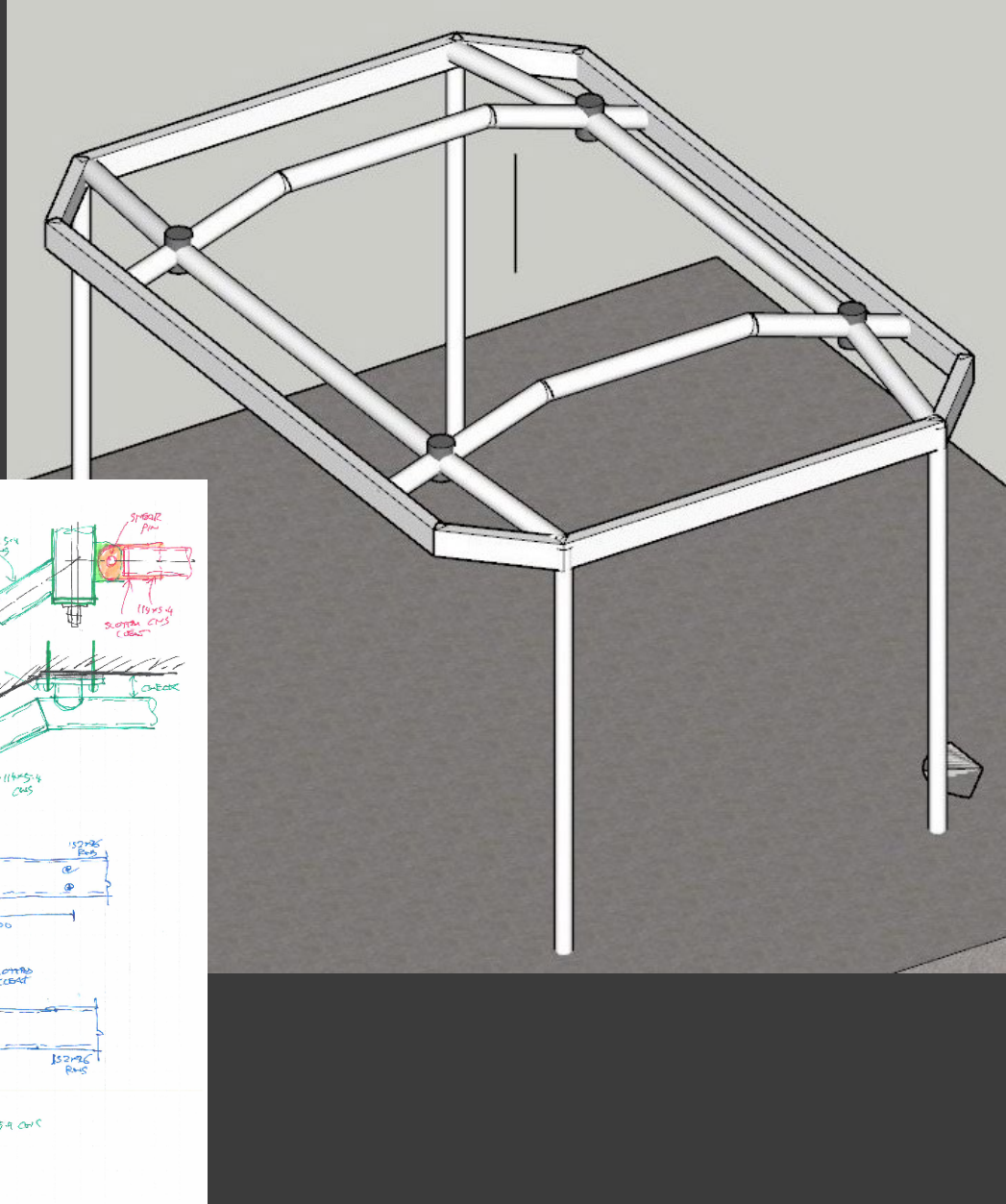
Case Study 4

Drying the void, testing concrete, fine tune proposal



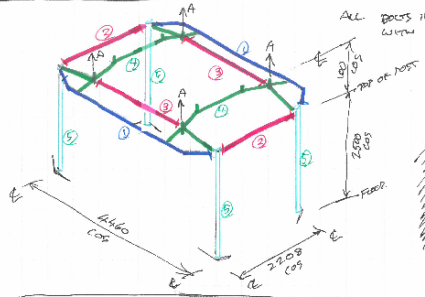
Case Study 4

Structural steel and Post-tensioning – simple on paper

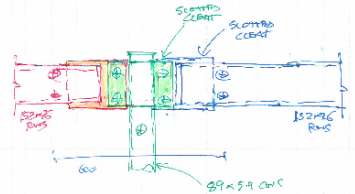
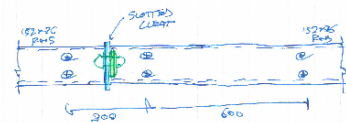
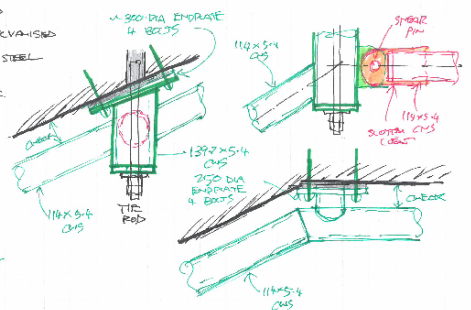
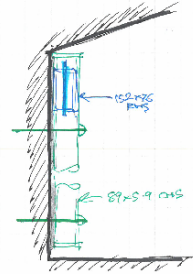
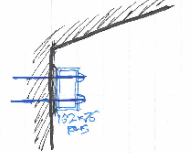
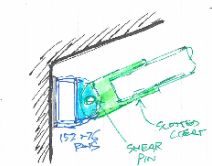
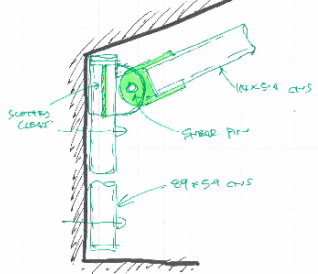


E19046G 2021/08/07
SCHEMATIC CRYPT STEEL DETAILS

ALL STEEL HDG600-5D PROTECTED
ALL STEEL TO STEEL CONNECTIONS GALVANISED
ALL BOSS INTO CEILING STAINLESS STEEL WITH ISOLATION
NO WELDING ON-SITE



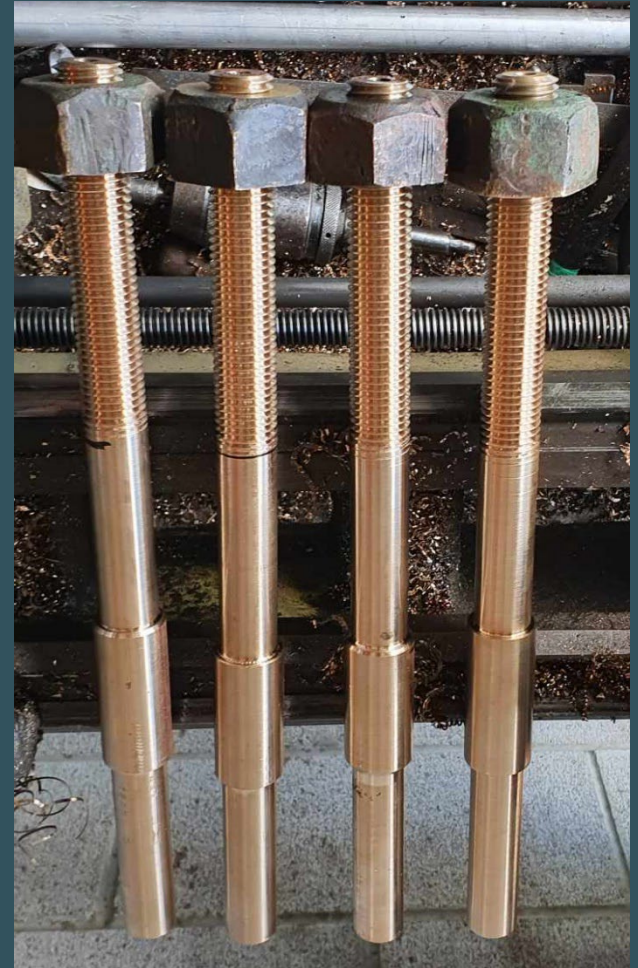
- LEGEND
- 1 TIE ROD
 - 2 152 x 76 x 6 RHS (2 OFF)
 - 3 152 x 76 x 6 RHS (2 OFF)
 - 4 114 x 54 CHS (2 OFF)
 - 5 COMBINATION - 114 x 54 CHS (2 OFF)
89 x 54 CHS (4 OFF)





Case Study 4

Stage 2 – Seismic Upgrade and Restoration



Case Study 4

Statue Fixings



Case Study 4

Coring the Column and the Iron Girder Issue



Case Study 4

Coring the Monument - 2 stages



Case Study 4

Coring – 8 vertical & 8 horizontal cores to monument



Case Study 4

Void Steelwork



Case Study 4

Installing the Crypt Steelwork



Case Study 4

Installing the Crypt Steelwork



Case Study 4

Completed ready for tensioning and replastering



Currently completing - cleaning, repointing, lead capping, replastering the Crypt, statue being reinstatement next week!

Case Study 4

Completion

Case Study 5: Wintergardens: Seismic upgrading (removal from EPB Register as a trial project) **triggering major replacement of fabric**

2015 Detailed Seismic Assessment Findings:

- Walls/piers – 67%NBS;
- Roof system excessively flexible and members insufficiently sized;
- Potential for cracking /shattering of brittle glass to roof – fall hazard;
- Tropical House chimney - potential fall hazard;
- Boiler House roof required a diaphragm;



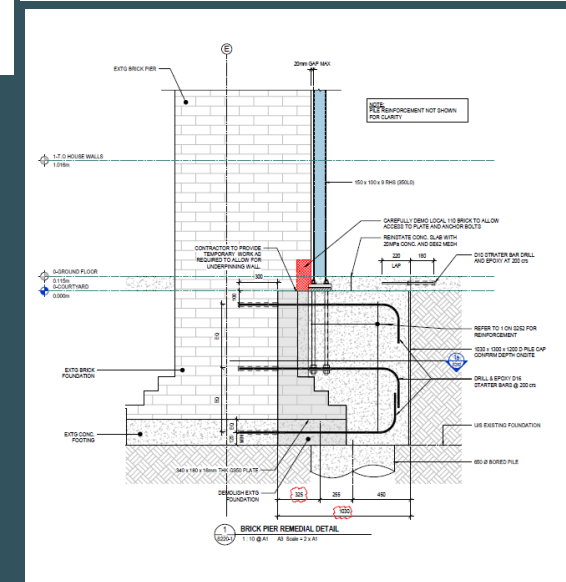
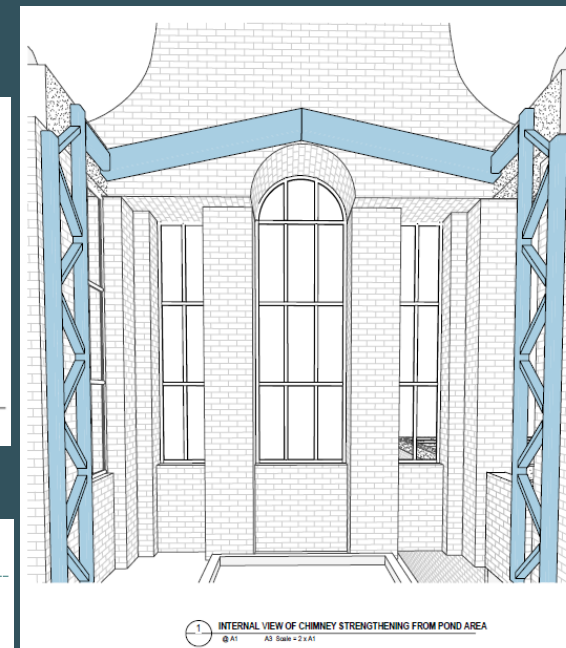
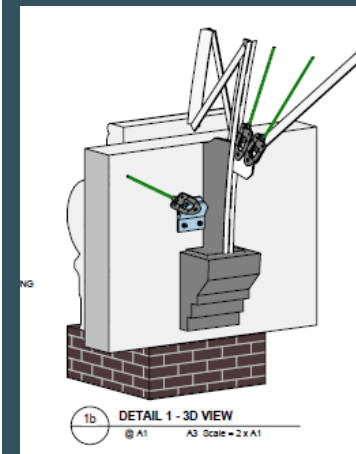
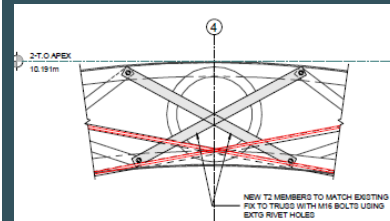
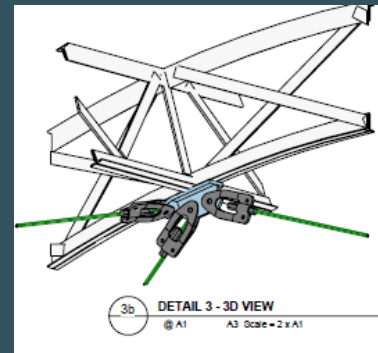
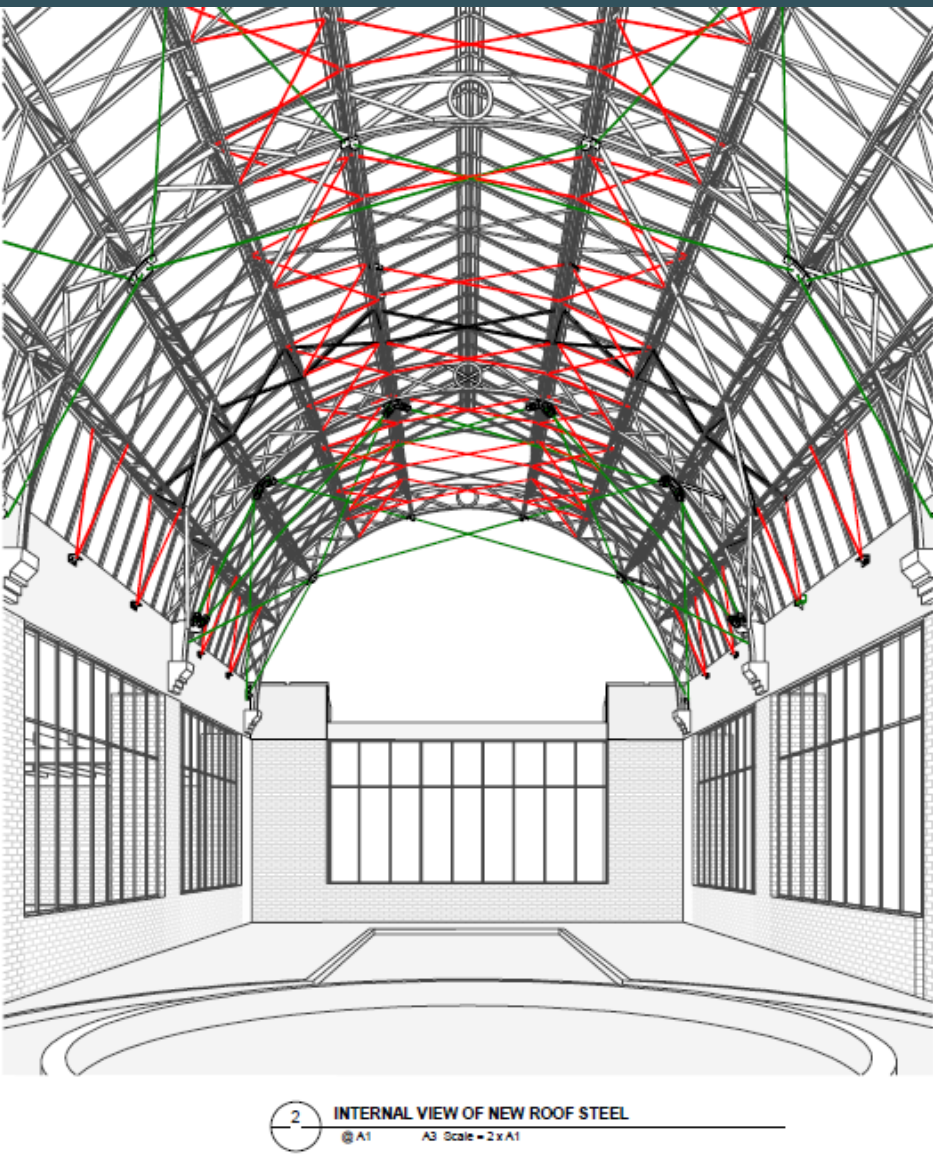
Recommendations:

1. Install diagonal tension bracing to the iron roof structure;
2. Supplement critical iron roof members;
3. Upgrade and underpin the chimney;
4. Add diaphragms to the Boiler House.



Case Study 5

Condition Report



Case Study 5

Consents obtained by the engineer and tendered



Case Study 5

Contractors queries about repairs and the glass

2017 - SPECIALIST GLAZING ENGINEER - ADVISED COUNCIL:

- Existing sloped roofs – 6mm toughened safety glass in AL bars - non-compliant – should be 8.76mm **heat strengthened laminated glass (HSL)**;
- Even if no live load allowance (for maintenance), min. requirement is 6.76mm **HSL glass** with glazing bars having **min. 13mm purchase**;
- All vertical glass above 5m and over fire exits - minimum 6.76mm **HSL glass with deeper rebates** to meet code;
- Replace glass and joinery should be replaced to accommodate movement or, provide catchment nets;

SERIOUS HEALTH AND SAFETY IMPLICATIONS –

Major replacement of glass and glazing bars to roofs, gables and porches at high level;

SALMOND REED APPOINTED AS LEAD CONSULTANT:

- Seismic retrofit;
- Major repairs;
- Glazing renewal;
- Improvements to the buildings;

Stage 1

2018/19 Contract - most urgent fabric repairs, without extensive scaffold;

2019 Finalise the design of the new glazing and review the seismic strengthening;

Stage 2

2019/20 Obtain new consents / tender documentation/ appoint contractor

Stage 3 – On site in 2 separate phases Temperate House first and then Tropical House

Stippolyte (obscure) - 2004/5



Originally clear glass - 1927

HSL Glass – 3mm layers of HS glass with an EVA film layer (double mechanical strength of annealed)

Supply issues:

1 overseas manufacturer of 3mm HSL Stippolyte

Clear glass readily available in NZ

ALUMINIUM GLAZING



New 6.76mm heat strengthened CLEAR laminated glass with new glazing bars.



New 8.76mm heat strengthened STIPPOLITE laminated glass with new glazing bars.

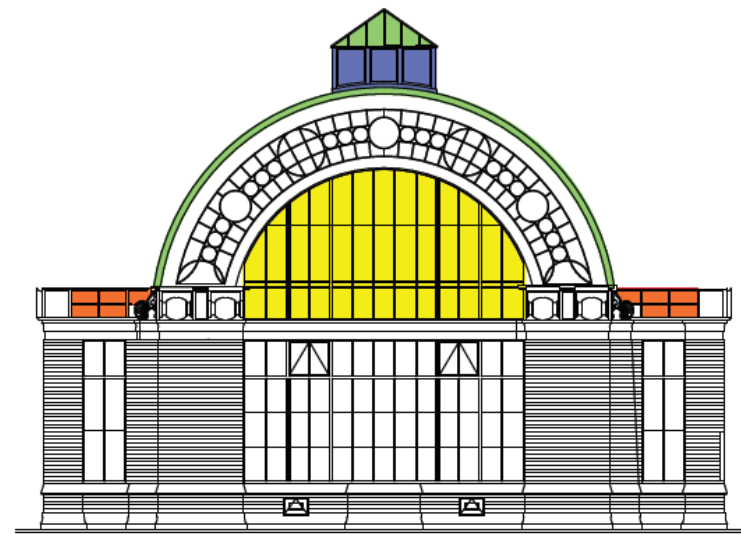
TIMBER GLAZING



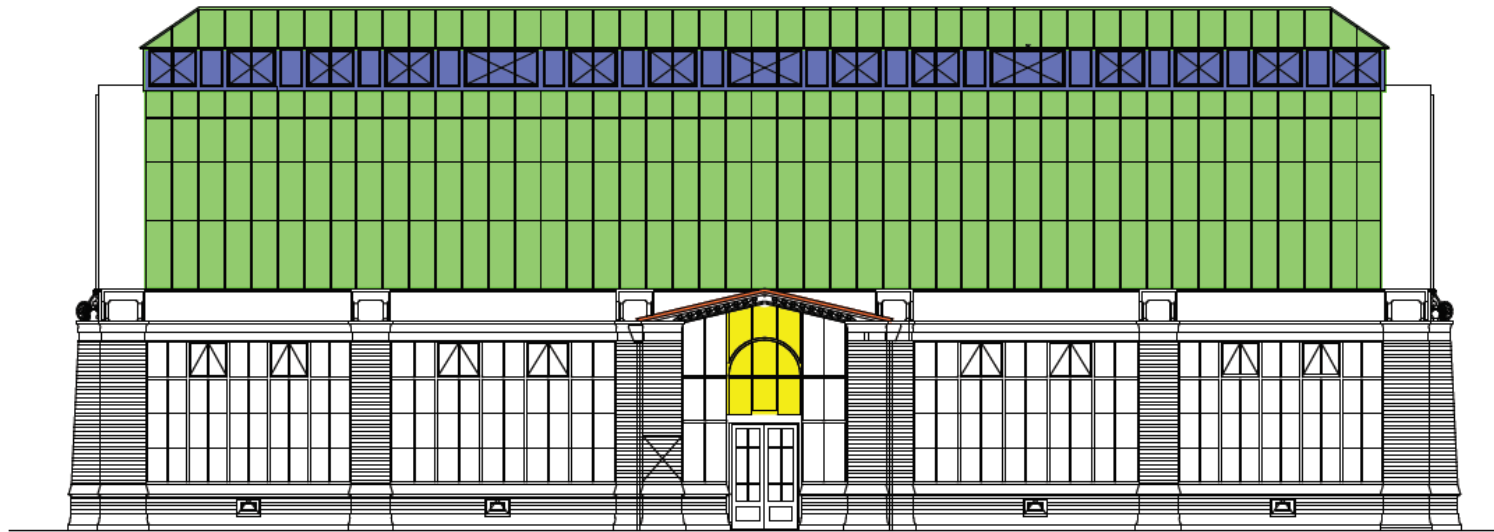
New 6.76mm heat strengthened CLEAR laminated glass with new rosewood beads.



New 6.76mm heat strengthened STIPPOLITE laminated glass with rosewood beads.



1 SOUTH ELEVATION



2 EAST ELEVATION



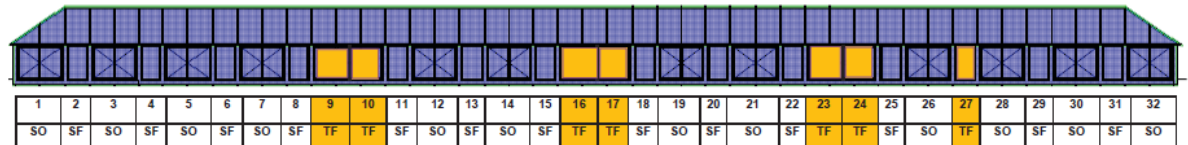
TROPICAL HOUSE - WEST ELEVATION LANTERN

TIMBER:

9 NARROW FIXED GLASS PANELS
6 WIDE FIXED GLASS PANELS

STEEL:

12 OPENING CASEMENTS
5 STEEL FIXED PANELS



TROPICAL HOUSE - EAST ELEVATION LANTERN

TIMBER:

1 NARROW FIXED PANELS
6 WIDE FIXED GLASS PANELS/ TIMBER

STEEL:

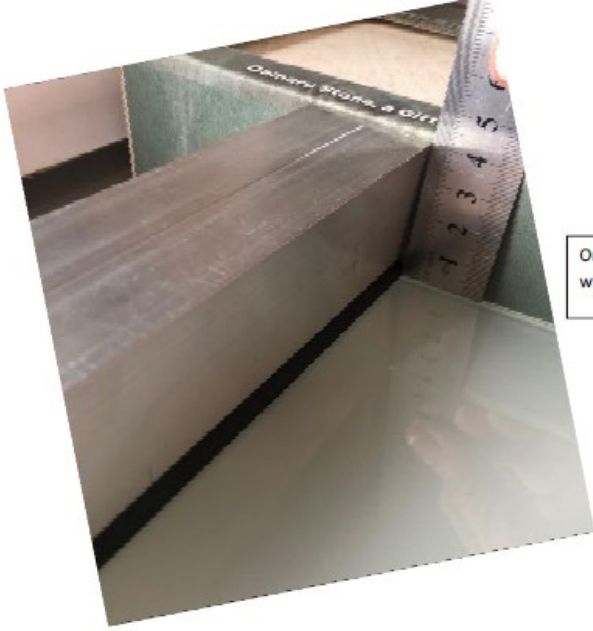
12 OPENING CASEMENTS
13 STEEL FIXED PANELS

LEGEND

- DENOTES TIMBER BEADS FIXED GLASS PANEL
- SF STEEL FIXED CASEMENT
- SO STEEL OPENING CASEMENT
- TF TIMBER FIXED CASEMENT

NORTH AND SOUTH ELEVATIONS:
6 STEEL FIXED CASEMENTS IN TOTAL

SRA/ SK/ 01 - TROPICAL HOUSE
LANTERN BEADING



Original Design of New Roof Glazing bar with gasket

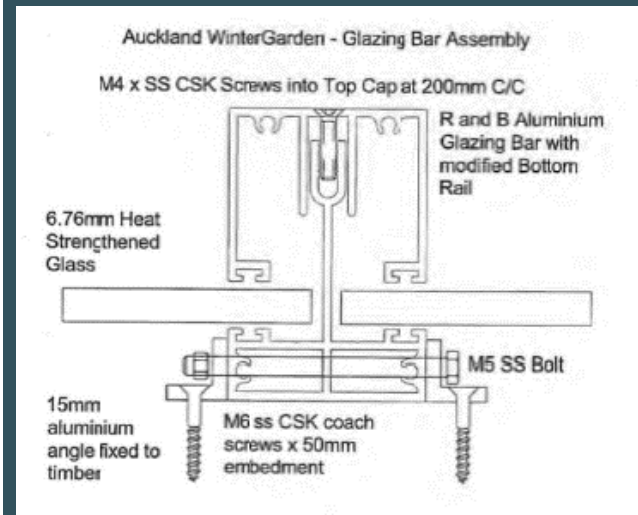
Original standard glazing 38mm high above glass



Re-designed, reduced height (reduced by 8mm in total when gasket in place)

New Roof Glazing bar – note gasket missing

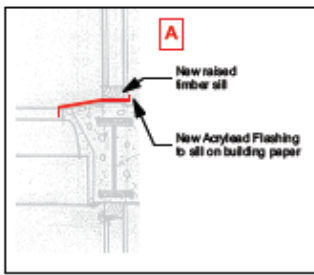
Redesigned to reduce the height of the bar by 8mm



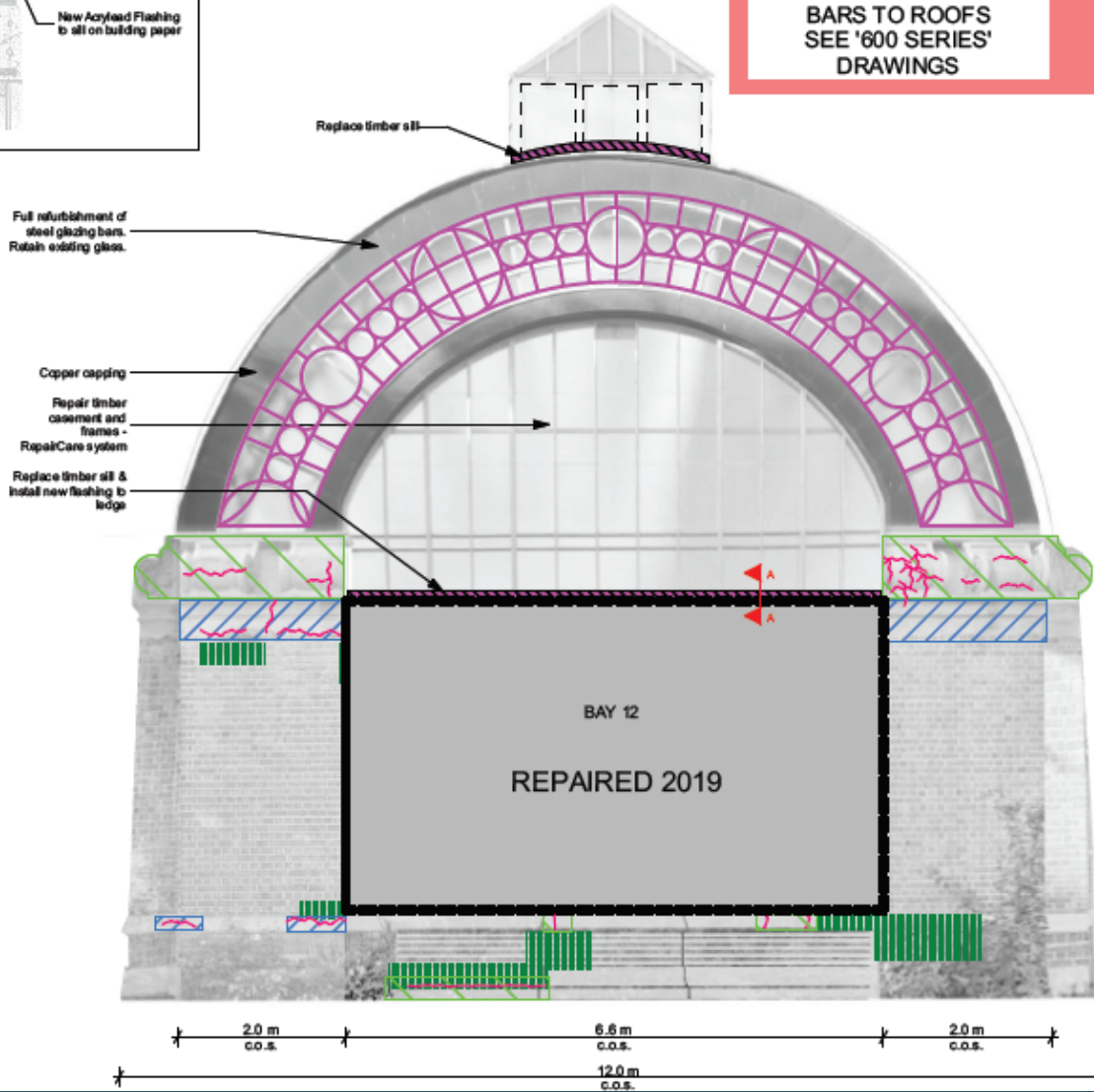
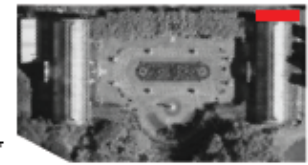


Case Study 5

Trial panel



SEISMIC UPGRADE:
FOR DETAILS OF NEW GLASS AND GLAZING BARS TO ROOFS SEE '600 SERIES' DRAWINGS



LEGEND	
	VENT
	OPENING TIMBER CASEMENT
	OPENING STEEL CASEMENT
SCOPE	
	DRILL OUT FIXING AND REPOINT
	REPAIR CRACK
	REMOVE VEGETATION AND MOSS
	REPLACE DAMAGED RENDER
	CLEAN SCABBLES AND REFACE CRAZED AND STAINED RENDER WITH NEW FINE'S COAT
	CLEAN IN ACCORDANCE WITH SPECIFICATION
	BRUSH EFFLORESCENCE
	REPOINT BRICKWORK
	REPLACE TIMBER SILL
	REFURBISH STEEL FRAME



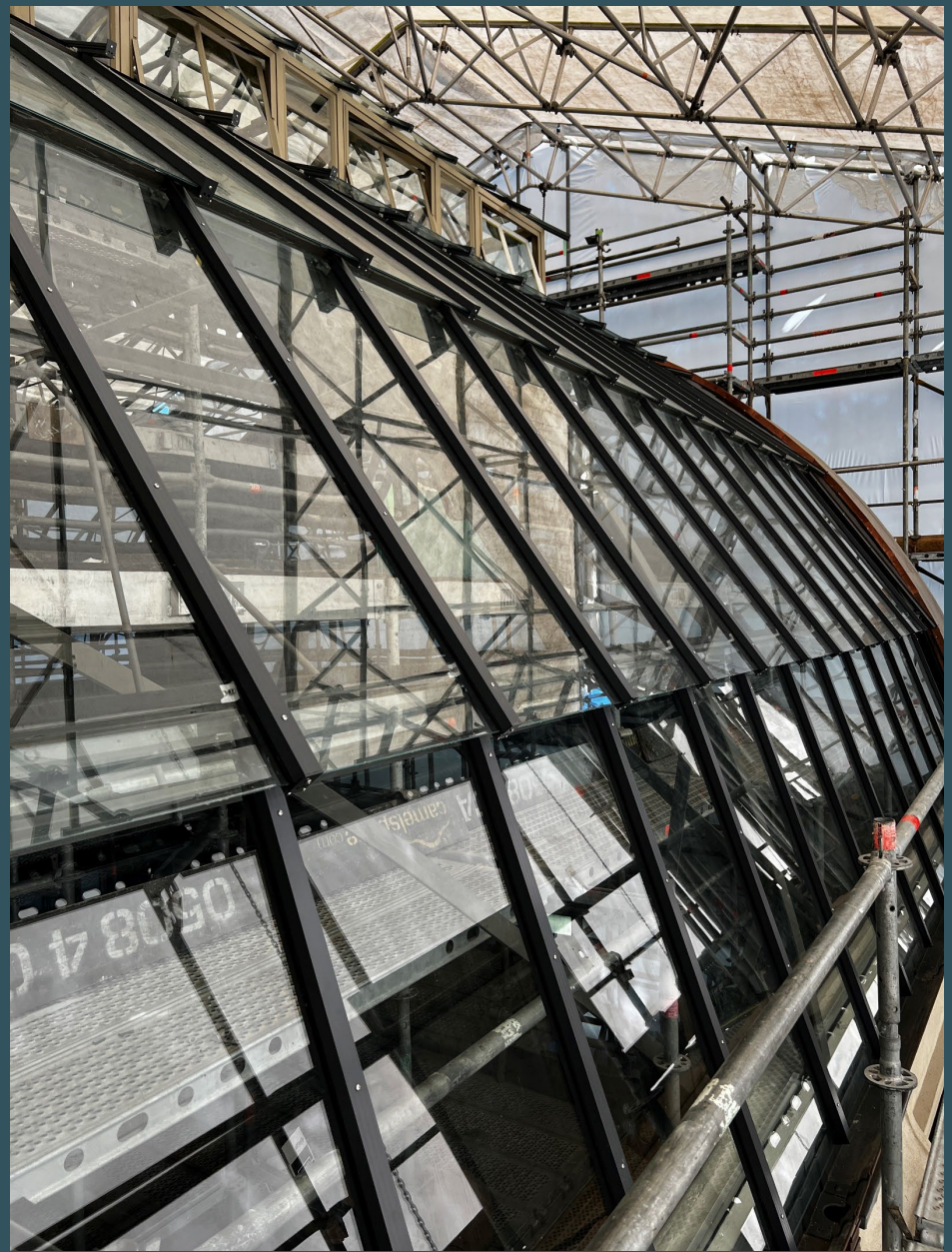
Case Study 5

Temperate House



Case Study 5

Temperate House



Case Study 5

Tropical House in progress – completion Feb 2023